

Study of Mechanical Properties of Concrete Using Construction and Demolition Waste in Concrete

Vinayak Ramdas Payghan

Assistant Professor, Dept. of Civil Engineering, Imperial College of Engineering and Research, Pune, India

Abstract: Recycling of construction and demolition rubble is not new concepts as several countries have been using crushed waste aggregates. It is estimated that the total waste generated in India is 960 million tones. The construction waste in India is estimated as 24 million tones, out of Brick and masonry waste is 3.60-4.40 million tones. These wastes create impacts on environment in the form of construction wastes debris. Impacts on environments can be minimized by finding various ways to reuse these products.

The present experimental investigation work describes possibility of using crushed masonry as aggregates. Herein experimental work clay bricks aggregate and cement bricks aggregates used as coarse aggregates at constant water cement ratio 0.45. In this project effort have been made to gain mechanical properties of concrete by inclusion of steel fiber. The steel fiber is added at systematically by 1%, 1.5% and 2% of cement content.

The comparative study is presented with respective to workability, compressive strength of M35 grade concrete. Recycled aggregates doing lesser workability compare with conventional concrete. Cement brick aggregates concrete obtained more strength compare with clay brick aggregates but both are gain lesser strength compare with conventional aggregates. It is found that, steel fibers taken more advantages to improve in strength of compare with conventional concrete.

Keywords: Concrete waste, Concrete waste reuse, Construction waste, New concrete.

1. Introduction

Concrete plays a vital part in our daily lives and in a functioning society. Concrete posses important property like durability, strength, high mould ability, structural stability, versatility, fire-resistance, Excellent thermal mass, Albedo effect and relatively low cost make it the backbone of buildings and infrastructure worldwide. It is estimated that the total solid waste generated in India is about 960 million tons of which the construction waste alone is 14.5 million tones. Construction and Demolition waste in India is estimated as 24 million tones out of which Bricks & masonry waste is 3.60 - 4.40 million tones. Impacts to the environment can be minimized by finding a way to reuse the products/materials when they reach their lifetime, which ensures only minimal impacts to environment, or alternatively, by extending their lifetime, thus improving their durability and reducing resource consumption.

Alternative materials such as CDW as well as other industries by-products are increasingly being tested and used as environmental sustainable natural aggregates substitutes. Rapid advances in Construction materials technology have enabled civil and structural engineers to achieve impressive gains in the safety, economy, and functionality of structure built to serve the common needs of society.

This study present an initial understanding of the current strengths and weaknesses of the practices intended to support construction industry in developing effect policies. It has been established that materials and components recovered from demolished buildings are being reused for new construction works as well as renovation projects, especially by low-income communities in developing countries.

Fiber use in concrete will increasingly continue to be the

preferred selection for many works like repair and rehabilitation projects involving constructions of tunnel, dams, bridges, Industrial floors, airport pavements, overlays, and high rise buildings, TV towers, parking garages, offshore structures, historic monuments and other structures. . The concept of using fibers as reinforcement is not new they have been used since ancient times to reinforce brittle materials. Horse hair was used to reinforce plaster, straws were used to reinforce sun baked bricks and asbestos fibers are being used to reinforce Portland cement. The applications of fibers in concrete are increasing rapidly. The fibers can be successfully used to improve the post cracking response of the concrete, i.e., to improve its energy absorption capacity and apparent ductility, and to provide crack resistance and crack control. The low tensile strength and brittle character of concrete have been bypassed by the use of fibers. Fiber may be an improved means of providing foundations for machinery where shock and vibratory loads are evident. Steel fiber reinforced concrete is a composite material consisting of a concrete matrix containing a random dispersion of steel fibers.

2. Materials

For preparation of concrete cube, clay brick aggregates concrete and cement brick aggregates concrete, materials used were cement, fine aggregates, coarse aggregates (20mm), coarse aggregates (10mm), Clay brick aggregates, Cement



brick aggregates, hooked end steel fibers, and water. Tests performed on materials used are discussed below with compression of IS codes.

A. Cement

The cement used in this experimental work was 53 Grade Ordinary Portland cement (OPC), and is used within one month of its arrival in the laboratory. IS: 12269-1987- Specification for 53 grades ordinary Portland cement [45] was referred to test all properties of cement. The properties of cement are listed in Table 1.

	Table 1				
	Properties of Cement				
S. No.	Tests Performed	Results			
1	Standard consistency.	34%			
2	Fineness	1.8%			
3	3 Specific gravity				
4	Setting time, a) Initial setting time	87 minute			
	b) Final setting time	255 minute			
5	Compressive strength at the age of a) 7 days				
	42.38 N/m				
	b) 28 days	56.57 N/mm ²			
6	Soundness test (Le- chatelier's)	1.0 mm			

B. Fine aggregate

Fine aggregates (sand) passing through 4.75 mm sieve confirming to IS: 383-1970 [49] was used. Natural sand from Godavari River at Paithan, District Aurangabad was used for experimental work. The quality and grading of fine aggregates, shown in Table 2. The sieve analysis was conducted to determine the particle size distribution in a sample of aggregate, which will call gradation. Sieve analysis of fine aggregates has been shown in Table 3.

Table 2 Physical Properties of Fine Aggregates Sr. No. Property Results Particle size 4.75 mm down 2 Particle shape Round 3 Fineness modulus 2.89 4 2.65 Specific gravity 5 Bulk density 1785 kg/m 6 Silt content 1.4% 7 Bulking of sand 4.18 %

Table 3					
	Sieve Analysis o	f Fine Aggregates			
IS Sieve size Weight Cumulative Cum			Cumulative		
	Retained gm	Weight	Percentage		
		Retained gm	Retained		
4.75 mm	22	16	1.6		
2.36 mm	109	131	13.1		
1.18 mm	110	241	24.1		
600 micron	365	606	60.6		
300 micron	295	901	90.1		
150 micron	92	993	99.3		
Lower than	7	-	-		
150 micron					
Total 1000		-	289.4		
Fin	Fineness Modulus (FM) = 289.4/100 = 2.89				

C. Coarse aggregates

Ph

Coarse aggregates are originated from basalt rocks. Different size of coarse aggregates was used in this experimental work. Aggregates size 20 mm down confirming to IS: 383-1970 [49] was used. Physical properties are shown in Table 4 and sieve analysis of coarse aggregates for size 20mm down shown in Table 5.

Table 4	
vsical properties of coarse aggregates	

Sr. No.	Property	Results
1	Particle Size	20 mm down
2	Particle Shape	Angular
3	Fineness Modulus of 20 mm	6.97
5	Specific gravity	2.81
6	Bulk density of 20 mm aggregate	1550 kg/m ³
7	Water absorption	1.3 %

Table 5					
	Sieve Analysia	s of Coarse Aggre	gates (Size 20 mm	down)	
S.	IS Sieve	Weight	Weight Cumulative Cumul		
No.	Size	Retained	Weight	Percentage	
		Kg	Retained kg	Retained	
1	40 mm	0	0	0	
2	20 mm	0.150	0.150	2.2	
3	10 mm	4.630	4.180	94.8	
4	4.75 mm	0.220	5.000	100	
5	2.36 mm	-	-	100	
6	1.18 mm	-	-	100	
7	600 micron	-	-	100	
8	300 micron	-	-	100	
9	150 micron	-	-	100	
Total	•	5.000	-	697	
Finenes	Fineness Modulus (FM) = 697/100 = 6.97				

D. Brick aggregates

Table 6

Sr. No.	Property	Results
1	Particle Size	20 mm down
2	Particle Shape	Angular
3	Fineness Modulus of 20 mm	6.84
4	Specific gravity	2.73
5	Bulk density of 20 mm aggregate	1800kg/m ³
6	Water absorption	7.25 %

 Table 7

 Sieve Analysis of Coarse Aggregates (Size 20 mm down)

Sr.	IS Sieve	Weight	Cumulative Weight Cumulat		
No.	Size	Retained	Retained kg Percenta		
		Kg		Retained	
1	40 mm	0	0	0	
2	20 mm	0.100	0.100	2.0	
3	10 mm	4.030	4.130	82.60	
4	4.75 mm	0.870	5.000	100	
5	2.36 mm	-	-	100	
6	1.18 mm	-	-	100	
7	600 micron	-	-	100	
8	300 micron	-	-	100	
Total		5.000		684	
Fineness Modulus (FM) $= 684/100 = 6.84$					

Brick aggregates are originated from burning of clay material at high temperature. In this experimental work clay brick



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Sr

6

aggregates were used collected from demolished construction work. Physical properties are shown in Table 6 and sieve analysis of brick aggregates for size 20mm down is shown in Table 7 for aggregates size 20mm down.

E. Cement aggregates (size 20 mm down)

Cement brick is combination of fine particles and cement. The use of cement brick is increased from last decades due to easy manufacturing process. In this experimental work cement brick aggregates were used collected from demolishing construction work. Table 8 indicates the physical properties and sieve analysis of brick aggregates for size 20mm down is shown in Table 9 for aggregates size 20mm down.

	Table 8				
Ph	ysical properties of cement coarse ag	gregates			
Sr. No.	Property	Results			
1	Particle Size	20 mm down			
2	Particle Shape	Angular			
3	Fineness Modulus of 20 mm	6.97			
4	Specific gravity	2.85			
5	Bulk density of 20 mm aggregate	1725 kg/m ³			
7	Water absorption	5.20 %			

Table 9						
	Sieve Analysis of Coarse Aggregates (Size 20 mm down)					
Sr.	IS Sieve	Weight	Weight Cumulative Cumul			
No.	Size	Retained	Weight Retained	Percentage		
		Kg	kg	Retained		
1	40 mm	0	0	0		
2	20 mm	0.100	0.100	2.0		
3	10 mm	4.030	4.130	82.60		
4	4.75 mm	0.870	5.000	100		
5	2.36 mm	-	-	100		
6	1.18 mm	-	-	100		
7	600	-	-	100		
	micron					
8	300	-	-	100		
	micron					
9	150	-	-	100		
	micron					
Total		5.000		684		
Fineness Modulus (FM) $= 684/100 = 6.84$						

F. Fibers

For the present experimental work DuraflexTM high tensile steel fibers with hooked ends are made from prime quality high carbon steel wire were used. They had hooked ends and were collated into clips of about10 individual fibers using water soluble adhesive as shown in Figure 1. The collation reduces the tendency for balling of fibers during the mixing process. The adhesive dissolved in the mixing water in about one minute, facilitating the distribution of individual fibers. These fibers are manufactured by the M/s Kasturi metal, Amravati, company with the trade name of DuraflexTM and type HKL80/60 BN.



Fig. 1. Hooked End Steel Fibers

	Table 10	
	Physical Properties of Fiber	
: No.	Description	Value
1	Type of fiber	Hooked end
		steel fiber
2	Length of fiber (<i>l</i>)	60 mm
3	Thickness (diameter) of fiber (d)	0.75 mm
4	Aspect ratio (<i>l</i> /d)	80



Tensile Strength

Specific gravity

Modulus of Elasticity

Photo-1: Hooked end Steel Fiber





Photo-3: Crushed Brick Aggregates



Photo-4: Cement Aggregates

3. Methodology

For the experimental work, following type of nine specimens for each mix designation were prepared. Out of nine cubes three are tested for 7 days compressive strength and remaining for 28 days for M35 grade concrete. Schedule of specimen preparation

>1100 N/mm²

7.8

200 Gpa



has been shown in Table 11.

~ .	Table 11							
Sch	edule	of Preparation of Specime	ens with	fiber co	ontent	w/c Ra	tio=0.4	45
	Sr. No.	Mix Designation	% of Brick aggregates replacement	% of Cement aggregates replace-	Fiber Content (V _f) %	Compression Test	Total Specimen	
	1	Conventional Concrete	0	0	0	9	9	
	2	$M35B_{40}F_0$	40	0	0	9	9	
	3	$M35B_{40}F_1$	40	0	1	9	9	
	4	$M35B_{40}F_{1.5}$	40	0	1.5	9	9	
	5	$M35B_{40}F_2$	40	0	2	9	9	
	6	$M35C_{40}F_0$	0	40	0	9	9	
	7	M35C ₄₀ F ₁	0	40	1	9	9	
	8	M35C ₄₀ F _{1.5}	0	40	1.5	9	9	
	9	$M35C_{40}F_2$	0	40	2	9	9	
		Total Cubes					45	

4. Mix design

The demolished concrete material is collected from 18 years old residential demolished building located in Aurangabad. The Demolished waste is crushed in concrete lab by hammer after crushing the material sizes are taken from sieve analysis. Mix proportion of concrete was decided by mix design. Mix design of concrete for grade M35 was done by using IS10262-2009[46]. The mix proportion of concrete for grade M 35 have 1: 2.05: 2.80: 0.45 (C: FA: CA 20mm: w/c). This thesis based on the partial replacement of normal aggregates by cement aggregates and brick aggregates. The natural aggregates are replaced by systematically 10%, 20%, 30% and 40%. The considerable strength reduction is observed for the 40% replacement, hence in this these the total coarse aggregates are used as 60:40 (NA: BA/CA) for new mix design.

Hooked end steel fiber are used as 1%, 1.5%, 2% of cement quantity and detail of steel fiber requirement for 432.00 kg cement is given in following Table 13.

Table 13 Steel fiber Ouantity in Kg				
Fiber dose in Percent Quantity in Kg				
1 4.32				
1.5	6.48			
2	8.64			

5. Results

6.

Compressive strength for different mix designations concrete has been calculated by equation and the results have mentioned in Table 14. Graph of failure compressive load with respect to fiber content and compressive strength with respect to fiber

Table 12

Mix proportion and quantity of brick aggregates and cement aggregates material in Kg for 1 CuM concrete

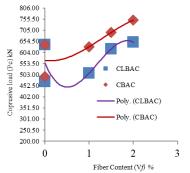
Cement (kg)	Fine aggregates (kg)	Aggregates (kg)					
		Normal aggregates		Brick aggregates		Cement aggregates	
		20mm	10mm	20mm	10mm	20mm	10mm
432.00	464.10	267.84	182.02	207.36	142.08	198.72	134.40

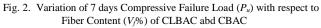
content has plotted as shown in Figure 2 to Figure .3 respectively for 7 days compressive strength.

Table 14
Compressive load (Pc), Compressive Strength (fcu) and Percentage increase
in Compressive Strength at 7 Days tests

in Compressive Strength at 7 Days tests						
L Sr. No.	Mix Designation	⁶⁰ Compressive load kN	$\stackrel{\text{\tiny L}}{\longrightarrow} \text{Comp. Strength } (f_{cu})$ MPa	^{Or} Variations in Comp. strength over Conventional Concrete	% increase in Comp. Strength over CC	
1	2	3	4	5	6	
1	$M35C_{40}F_2 M35C_{40}F_{1.5} M35C_{40}F_1 M35C_{40}F_1 M35B_{40}F_2 M35B_{40}F_{1.5} M35B_{40}F_1 M35B_{40}F_0 Conventional M35C_{40}F_2 M35B_{40}F_1 M35B_{40}F_1 M35B_{40}F_0 Conventional M35C_{40}F_1 M35B_{40}F_1 $	640.25	28.46	AA	NA	
7	$M35B_{40}F_0$	470.0	209	-7.53	-26.45	
3	$M35B_{40}F_1$	510.15	22.6	-5.78	-20.32	
4	$M35B_{40}F_{1.5}$	620.37	27.55	-0.88	-3.11	
5	M35B ₄₀ F ₂	650.90	28.9	0.47	1.66	
6	$M35C_{40}F_0$	495.22	22.0	-6.45	-22.65	
٢	$M35C_{40}F_1$	630.15	28.0	-0.45	-1.58	
×	$M35C_{40}F_{1.5}$	695.10	30.8	2.44	8.57	
6	$M35C_{40}F_2$	751.22	33.39	4.93	17.33	







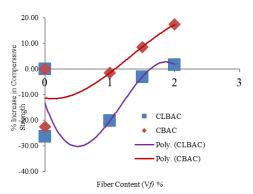


Fig. 3. Variation of Percentage in 7 days Compressive Strength (f_{cu}) with respect to Fiber Content (V_{f} %) of CLBAC abd CBAC



Photo-5: Compression testing machine after placing specimen



Photo-6: Failure of Specimen

Compressive strength for different mix designations for 28

days compressive strength shown in table.

Table 15

Comj	Table 15 Compressive load (Pc), Compressive Strength (fcu) and Percentage increase in Compressive Strength at 28 Days tests						
Sr. No.	Mix Designation	Compressive load kN	Comp. Strength (f_{ca}) MPa	Variations in Comp. strength over Conventional Concrete	% increase in Comp. Strength over CC		
1	2	3	4	5	6		
1	M35B ₄₀ F ₀ Conventional Concrete	910.50	40.47	NA	NA		
2	M35B ₄₀ F ₀	709.85	31.55	-8.92	-22.04		
ю	M35B ₄₀ F ₁	777.86	34.57	-5.90	-14.57		
4	$M35C_{40}F_2$ $M35C_{40}F_{1.5}$ $M35C_{40}F_1$ $M35C_{40}F_0$ $M35B_{40}F_2$ $M35B_{40}F_{1.5}$ $M35B_{40}F_1$	927.55	41.22	0.76	1.87		
5	M35B ₄₀ F ₂	945.60	42.03	1.56	3.86		
9	M35C40F0	715.20	31.79	-8.68	-21.45		
٢	M35C40F1	904.50	40.20	-0.27	-0.66		
∞	$M35C_{40}F_{1.5}$	1010.50	44.91	4.44	10.98		
6	M35C ₄₀ F ₂	1070.00	47.56	7.09	17.52		

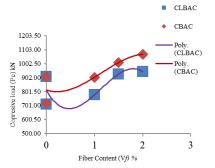


Fig. 4. Variation of 28 days Compressive Failure Load (P_u) with respect to Fiber Content $(V_{1}\%)$ of CLBAC and CBAC



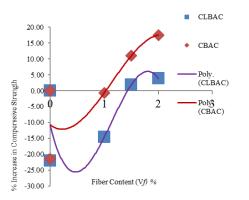


Fig. 5. Variation of Percentage in 28 days Compressive Strength (f_{cu}) over Conventional Concrete with respect to Fiber Content (V_f %) of CLBAC and CBAC

The above results are discussed with respect to the results obtained the compressive Strength of the cubes under the 7, 28 days curing.

7. Conclusion

An experimental study of suitability of clay brick and cement brick aggregate with steel fiber has been carried out in the laboratory. On the basis of results obtained from experimental work, observation made during casting and testing of conventional concrete, clay brick and cement brick aggregate concrete, the following conclusion are drawn:

- 1) Workability of concrete decreases as rapidly as steel fibers content increases, without changing any water cement ratio, water content and cement content.
- 2) Brick aggregates concrete is more workable as compare to the cement brick aggregates.
- 3) There was a systematically decrease in compressive strength, flexural strength and split tensile strength with incorruption of 40% of brick aggregates and cement brick aggregates. However, the incorruptions up to 40% of recycled masonry aggregates to partially replace in conventional concrete, caused a decrease in all strength parameter.
- 4) Maximum loss in compressive strength of brick aggregates concrete and cement aggregates concrete is 22.04% and 21.45% for 0% fiber content and maximum compressive strength for 2% for content is 3.86% and 17.52% as compared conventional concrete.
- 5) Cement aggregates concrete is more suitable for concrete as compare to brick aggregates concrete in all strength parameter.
- 6) The optimum fiber content is 2% compare with 1% and 1.5% and 2% fiber content. The results revels that higher fiber content has brought in increased compressive strengths, flexural strengths, split tensile strength and crack control effect.

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